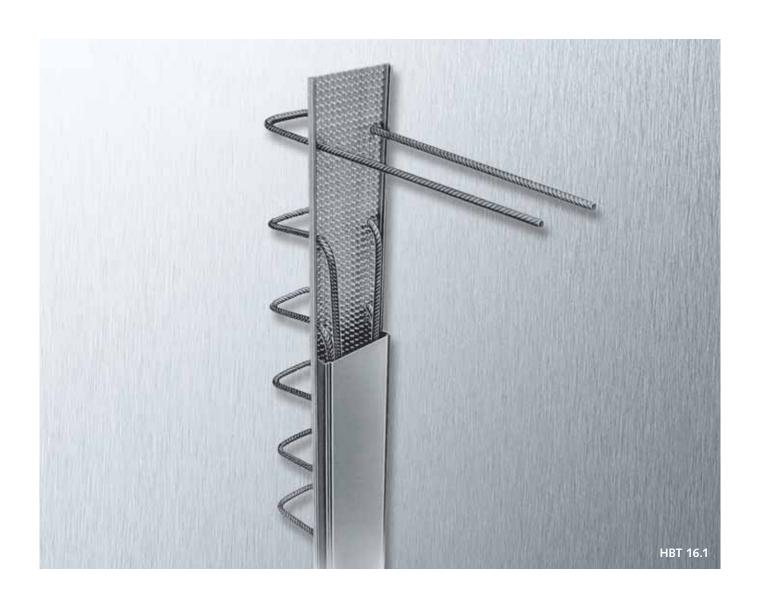




# HALFEN HBT REBEND CONNECTION Technical Product Information



- DIBt Approval Z-21.8-2035
- Type tested acc. to approval and EUROCODE 2



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CAD drawings for HBT-Elements can be found in our CAD library under reinforcement technology. All drawing can be downloaded free at www.halfen.com. Following file formats are available:

• CAD - DWG and DXF

A free DVD is also available if preferred. Our contact details, addresses, telephone and fax numbers can be found on the back cover of this catalogue Following approval and type test apply in this document:

- German National Technical Approval (DIBt Deutsches Institut f
  ür Bautechnik) No. Z-21.8-2035 for HALFEN HBT Rebend Connection
- Type test report no. 4117-6131/14 for HALFEN HBT Rebend Connection according to approval no. Z-21.8-2035

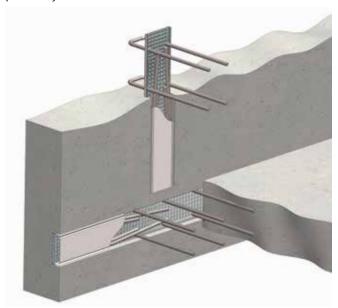


2

Introduction and System Description

#### HBT - the connection solution

The HALFEN HBT is the first rebend connection with general building approval. The simplified calculation method according to the approval requires verification of only two basic load cases, this results in higher shear load capacity than previously.



#### Safety

- improved planning-reliability through general building authority approval based on real-world load capacity tests
- type tested load capacity tables
- more safety in planning and execution due to factory production and third party control

#### Simple

- simplified calculation concept with only two basic load cases - shear load longitudinal to joint

  - shear load transverse to joint
- both shear and longitudinal load transfer using standard type profiles
- if required a verification of combined shear and longitudinal load in the concrete joint is possible
- standard type is suitable for constructive connections as well as for static critical connections - no risk of mix ups on-site or in the precast plant
- reduced case height ideal for thin elements or precast concrete elements with minimal concrete cover

HALFEN HBT Rebend connections allow a simple and effective connection of reinforced concrete elements, which are cast in different phases. All types of slabs, from floor slabs, walls and stairs can be subsequently cast with sufficient bond.

The rebar are bent and secured in a case with a back and a removable cover. Our profile cases are available in different widths. The case is cast into the concrete element: after striking the formwork, the case cover is removed and the rebar is straightened.

The customer can select either a single-row or double-row profile. The single-row profile has a regular spacing of holes in a single row penetrated by reinforcement bars designed for the required application; the double-row profile is similar but has two rows.

- rebar (8, 10, 12 mm) B500B steel (stainless steel B500 NR on request)
- the back is galvanised sheet metal with a specially profiled surface
- dimensionally stable, galvanised, sheet metal cover with a pre-punched hole to facilitate removal



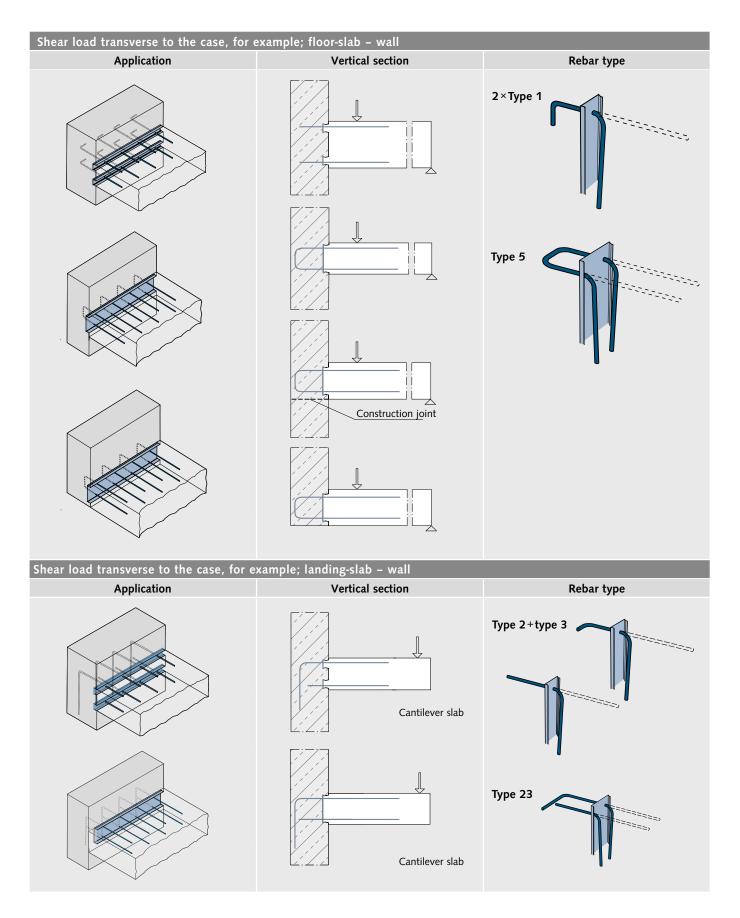
#### Versatile

Optimal connections for a multitude of applications with 57 possible combinations of rebar and profile widths. A standardized range of product with element lengths of 0.8 m and 1.25 m are available for the most common applications.

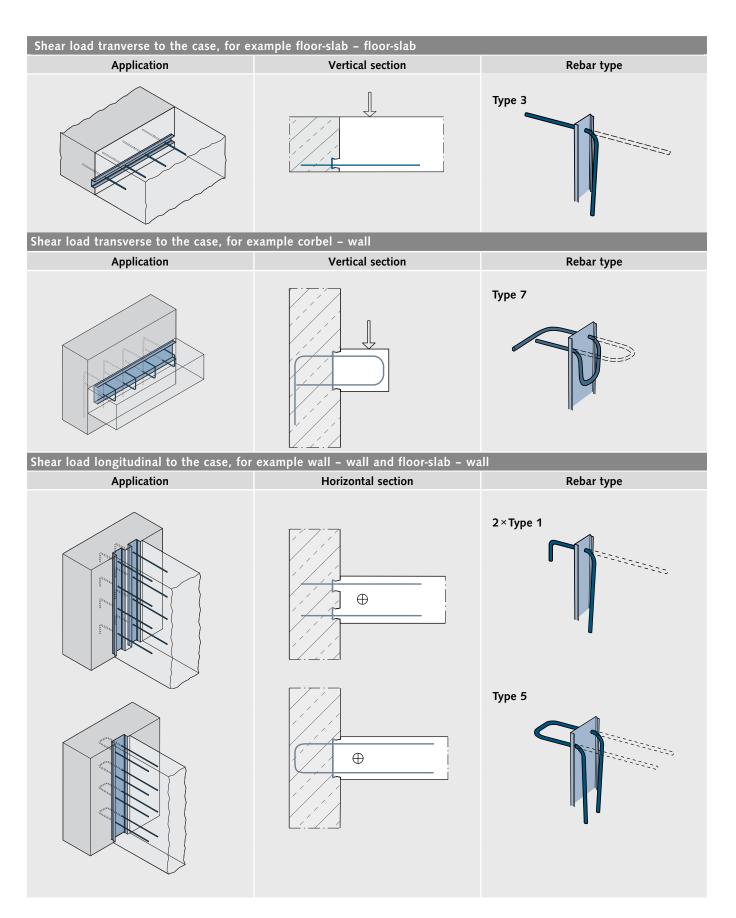
Lots of reasons, one conclusion: safety, quality and protection - for you and your company.



# Type Overview/Application Examples

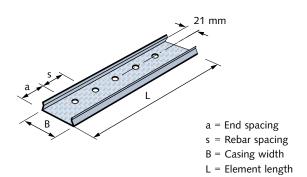


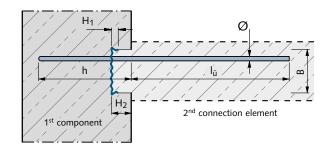
# Type Overview/Application Examples



Single-Row Profiles

# Profile dimensions



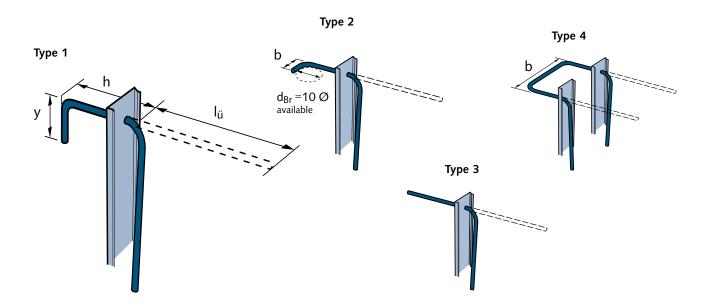


Case dimensions [mm]									
profile	rebar Ø [mm]	width B	height H <sub>1</sub>	height H <sub>2</sub>					
HBT 55	8	58		24					
HRI 22	10	90		30					
HBT 85	10	86	12	30					
כס ומח	12	00	12	36					
HBT 120	10	122		30					
1101 120	12	122		36					

Rebar layout			
element length L	rebar spacing s [cm]	number of rebar	end spacing a [cm]
	10	12	7.5
standard element	15	8	10.0
L = 1250 mm	20	6	12.5
	25	5	12.5
	10	8	5.0
short element	15	6	2.5
L = 800 mm	20	4	10.0
	25	4	2.5

Other element lengths on request

# Rebar types



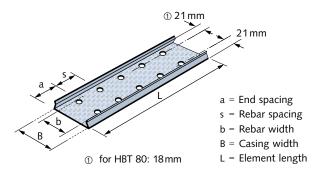
See table on page 7 for rebar dimensions

Single-Row Profiles

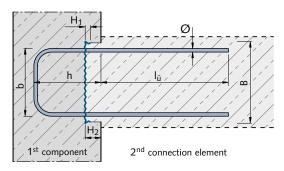
Rebar dimensior	ıs [mm]								
profile	rebar Ø	rebar spacing		standard type 1			types 1, 2, 3, 4		only for type 4
		8		L = 1250 mm			L = 1250 mm	L ≥ 800 mm	
	[mm]	[cm]	h	lü	у	h <sub>min</sub>	I <sub>ü,max</sub>	I <sub>ü,max</sub>	b [mm]
HBT 55		10		210			210	210	
	0	15	170		75	120	510	430	
	8	20	170	320		120	600	455	
		25					600	480	200500
		10		200		120	200	200	200500
	10	15	170	390	95		390	390	
	10	20					510	450	
		25						475	
HBT 85		10	170	390	95	120	430	400	
	10	15					510	425	
	10	20	170				600	450	
		25						475	250500
		10		430		110 120	430	395	
	12	15	170		110		510	420	
	12	20	170	460			600	445	
		25					000	470	
HBT 120		10						400	
	10	15	170	390	95	120	600	425	
	10	20	170	350	23	120	000	450	
		25						475	on request
		10						395	
	12	15	170	460	110	120	600	420	
	12	20	170	100				445	
		25						470	

#### **Double-Row Profiles**

# Profile dimensions



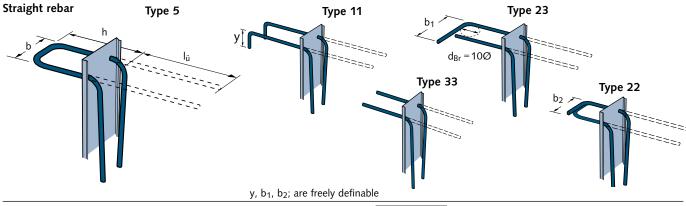
Case dimer	Case dimensions [mm]									
profile	rebar Ø [mm]	width B	height H <sub>1</sub>	height H <sub>2</sub>	b					
HBT 80	8	86		24	58					
пвт во	10	00		30	60					
	8			24	88					
HBT 120	10	122		30	90					
	12		2		36	92				
	8			24	116					
HBT 150	10	150	12	30	118					
	12		12	36	120					
	8			24	152					
HBT 190	10	186		30	154					
	12			36	156					
	8			24	188					
HBT 220	10	222		30	190					
	12			36	192					

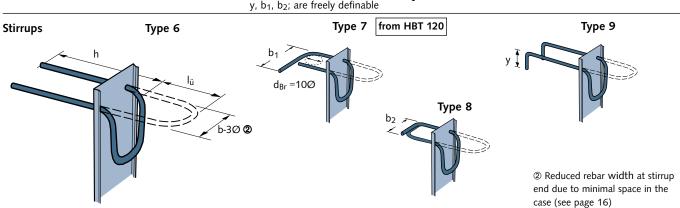


Rebar layout				
element length L	rebar spacing s [cm]	number of rebar	end spacing a [cm]	
	10	12	7.5	
standard element	15	8	10.0	
L = 1250 mm	20	6	12.5	
	25	5	12.5	
	10	8	5.0	
short element	15	6	2.5	
L = 800 mm	20	4	10.0	
	25	4	2.5	

Other element lengths on request

# Rebar types





# Double-Row Profiles

profile	rebarØ	rebar	standard	type 5	type	es 5, 11, 22, 23	4). 33		types 6,	7. 8. 9	3, 9							
prome	TCDar	spacing	L=1250 mm	type 5	type	L=1250 mm		type 6	type 7 ③, 8	type 9	L≥ 600 mm							
	[mm]	[cm]	h	I.							60≤l <sub>ü</sub> ≤l <sub>ü,ma</sub>							
	[]		- 11	l <sub>ü</sub>	h <sub>min</sub>	l <sub>ü,max</sub>	I <sub>ü,max</sub>	h <sub>min</sub>	h <sub>min</sub>	h <sub>min</sub>								
HBT 80 ③		10		170		170	170				70							
	8	15	170	220	120	360	360	120	135	140	125							
		20		320		450	450				175							
		25		460		500	480				225							
		10		160		160	160				60							
	10	15	170	320	120	320	320	120	155	140	120							
		20		390		400	400				170							
		25		200		450	450				220							
HBT 120		10		290		290	290				115							
	8	15	170		120	510	430	120	135	140	175							
		20		320		600	455				235							
		25					480				290							
		10		240		240	240		) 155		80							
	10	15	170		120	510	425	120		140	130							
		20		390	120	600	450		. 33		180							
		25					475				230							
		10		215		215	215				70							
	12	15	170	390	120	390	390	120	170	140	120							
	'-	20	170	440	120	440	440	120	1, 0	110	170							
		25		460		490	470				220							
HBT 150		10				360	360					100						
	8	15	170	320	120	510	430	120	135	140	150							
	0	20	170	320	120	600	455	120	133	140	210							
		25				000	480				260							
		10		360	360	360	360				85							
8	10	15	170		120	510	425	120	155	140	135							
	10	20	170	390	120	600	450	120		140	185							
		25				600	475				235							
		10		310		310	310				90							
	12	15	170		120	480	420	120	170	140	150							
	12	20	170	460	120	530	445	120	170	140	200							
		25				580	470				250							
HBT 190		10				500	405				100							
	0	15	470	220	420	510	430	420	425	4.40	150							
	8	20	170	320	120	600	455	120	135	140	210							
		25				600	480				260							
		10				500	400				110							
	4.0	15	470	222	400	510	425	400	455		170							
~	10	20	170	390	120		450	120	155	140	220							
		25				600	475				280							
		10		430		430	395				90							
		15				510	420				140							
	12	20	170	460	120		445	120	170	140	190							
		25				600	470				240							
HBT 220		10					405				100							
1101 220		15					430				150							
	8	20	170	320	120	600	455	120	135	140	210							
		25					480				260							
		10					400				110							
							425				170							
2	10 15	20	170	390	120	600	450	120	155	140	220							
		25		-50			475				280							
		10					395				90							
											140							
	12	15	170	460	120	600	420	120	170	0 140								
	12 20	1)	20	20 25		1)	1)	20	20	170	400	120	600	445		1, 0	140	190

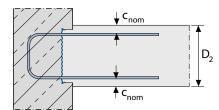
@ 2020  $\cdot$  HBT 16.1-E  $\cdot$  www.halfen.com 9

 $<sup>\</sup>textcircled{3}$  rebar type 7 for HBT 80 not available 4 Due to the required bending roll diameter (d<sub>Br</sub> = 10x bar diam.); h<sub>min</sub> for type 23 is equal to h<sub>min</sub> of type 7

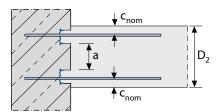
Product Selection/Calculation Basis according to Approval no. Z-21.8-2035

Product selection allo	Product selection allowing for required concrete cover of the rebend reinforcement											
eu.	Thickness of component D <sub>2</sub> [mm]											
profile	100	120	140	160	180	200	220	240	260	280	300	
	concrete cover c <sub>nom</sub> [mm]											
2×HBT 55 ①	-	-	≤ 25	≤ 35	≤ 45	≤ 55	≤ 65	≤ 75	≤ 85	≤ 95	≤ 105	
2×HBT 85 ①	-	-	-	-	≤ 20	≤ 30	≤ 40	≤ 50	≤ 60	≤ 70	≤ 80	
HBT 80	20	30	40	50	60	70	80	90	100	110	120	
HBT 120	-	-	25	35	45	55	65	75	85	95	105	
HBT 150	-	-	-	20	30	40	50	60	70	80	90	
HBT 190	-	-	-	-	-	23	33	43	53	63	73	
HBT 220	-	-	-	-	-	-	-	25	35	45	55	

① depends on spacing a between the cases



Product selection for 1-part element



Product selection for multi-part elements

#### Basis for calculation according to approval number Z-21.8-2035

# General information

The concept for the approval is based on the calculation and the structural application as applied in the following standards and guidelines: DIN EN 1992-1-1 with DIN EN 1992-1-1/NA (National Annex) and the DBV-guidelines "Rebending of reinforcing steel and requirements of protective boxes according to Eurocode 2". Generally two different cases of shear load are examined: shear load transverse and longitudinal to the concrete joint.

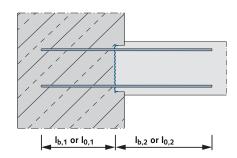
According to the approval, due to the product characteristics the shear loads transverse to the joint in the HBT Rebend connections can be classed as "indented" and shear loads longitudinal to the joint can be classified as "rough".

Material: Rebend reinforcement B500B, stainless steel
B500 NR with bar diameters of 8, 10 and 12 mm
Normal concrete ≥ C20/25

The maximum load bearing capacity of the rebend connection must be limited to 80% of the ultimate limit state; the following applies for tensile strength:

$$f_{yd,red} = 0.8 \cdot \frac{f_{yk}}{\gamma_s}$$

Existing anchorage lengths and overlap lengths must be taken into consideration for the calculation. These can be calculated from the back surface of the case. Verification of the anchorage lengths and overlap lengths is according to Eurocode 2, taking the bonding characteristics into account.



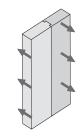
Anchorage and overlap length requirements for the reinforcement

Calculating the HALFEN HBT Rebend connections as in the DBV-guidelines "Rebending of reinforcing steel and requirements of protective boxes according to Eurocode 2" with roughness classification "smooth", is conservative while still being acceptable.

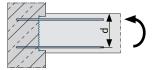
Calculation Basis according to Approval no. Z-21.8-2035

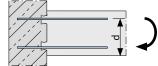
#### Shear load, transverse to the concrete joint

Calculation is according to DIN EN 1992-1-1, section 6.2 and DIN EN 1992-1-1/NA, as for monolithic produced building components; whereby the following additional provisions must be observed.



Effective static height:

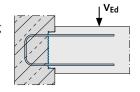




Tension zones: upper and lower component edges

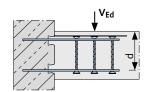
#### Shear resistance with no shear reinforcement

The decisive resistance  $V_{Rd,c}$  for the calculation is according to DIN EN 1992-1-1, section 6.2.2; whereby a reduction in the reinforcement ratio  $\rho_l$  is not required (caused by the reduced yield strength of the rebend reinforcement).



#### Shear resistance with shear reinforcement

The decisive resistance for verification results from 30% of the shear load resistance V<sub>Rd,max</sub> according to DIN EN 1992-1-1, section 6.2.3:



 $V_{Ed} \le V_{Rd} \le 0.3 \cdot V_{Rd,max}$ 

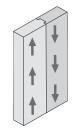
An additional load in the longitudinal reinforcement caused by shear forces must be verified assuming a compression strut angle of 45° (cot  $\theta$  = 1.0). The required shear reinforcement is calculated using cot  $\theta$  = 1.0

To calculate the shear load reinforcement for HALFEN HDB-S Shear rails see HDB technical product information and the approvals Z-15.1-249 and Z-15.1-270.

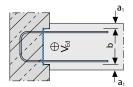
# Shear load, longitudinal to the concrete joint

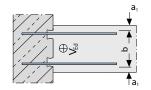
Static verification is according to DIN EN 1992-1-1, section 6.2.5 and DIN EN 1992-1-1/NA (National Annex), whereby the following additional provisions are to be observed. Factors to used to calculate c = 0.4

Factors to used to calculate the shear load resistance:



 $\mu = 0.7$ v = 0.5





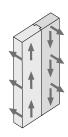
If the spaces between the HBT Cases are additionally subjected to shear load, then these must be designed as rough or suitably indented in accordance with DIN EN 1992-1-1. The edge areas can also be assumed as load bearing if  $a_1 \ge 50 \, \text{mm}$ .

The concrete cover  $c_{nom}$  is according to DIN to EN 1992-1-1. In addition for the rebend reinforcement the following must be observed.

c<sub>nom</sub> ≥ max. {3Ø, 30 mm, max. aggregate diameter d<sub>g</sub>}

#### Combined shear load, transverse and longitudinal to the concrete joint

If the connection is subjected to combined shear load (longitudinal and transverse to the concrete joint), each load direction can be verified separately.



#### TECHNICAL CONSULTATION

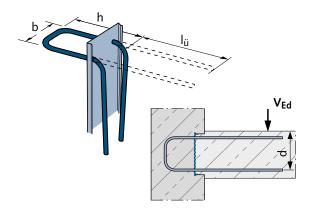
#### **Technical services**

See inner back of catalogue for technical advice for your individual projects and contact information for all products.

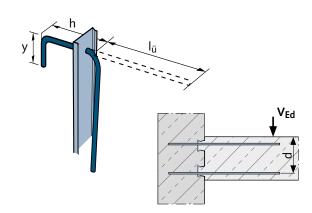
Selected Load Capacity Values according to Type Test and Approval no. Z-21.8-2035

# Shear load capacity transverse to the case — no shear reinforcement

Single connection example;  $1 \times Type 5$ 



Multipart connection example; 2 × Type 1



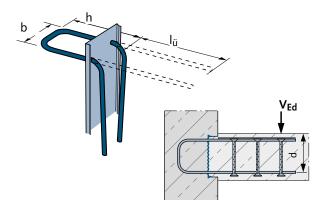
Shear load resistance $V_{Rd}$ [kN/m] ( $\leq 0.3 \cdot V_{Rd,max}$ ) Always refer to the information in the type te										n the type test
rebar Ø/	d = 100 mm	d = 120 mm	d = 140 mm	d = 160 mm	d = 180 mm	d = 200 mm	d = 220 mm	d = 240 mm	d = 260 mm	d = 280 mm
spacing s [mm/cm]	HBT 80	HBT 120	HBT 150	HBT 150	HBT 190	HBT 190	HBT 220	HBT 220	2×55(Ø8) 2×85(Ø10,12)	2×55(Ø8) 2×85(Ø10,12)
concrete strength class C20/25										
8/20	44.3	53.1	62.0	70.8	78.7	78.7	78.7	78.7	78.7	78.7
8/15	44.3	53.1	62.0	70.8	79.7	88.5	94.0	99.4	104.7	104.9
8/10	-	53.1	62.0	70.8	79.7	88.5	94.0	99.4	104.7	109.8
10/20	44.3	53.1	62.0	70.8	79.7	88.5	94.0	98.4	98.4	98.4
10/15	44.3	53.1	62.0	70.8	79.7	88.5	94.0	99.4	104.7	109.8
10/10	-	56.6	62.7	70.8	79.7	88.5	94.0	99.4	104.7	109.8
12/20	-	53.1	62.0	70.8	79.7	88.5	94.0	99.4	104.7	109.8
12/15	-	55.8	62.0	70.8	79.7	88.5	94.0	99.4	104.7	109.8
12/10	-	63.9	70.8	77.4	83.7	89.8	94.0	99.4	104.7	109.8
				concrete	strength cla	ss C25/30				
8/20	49.5	59.4	69.3	78.7	78.7	78.7	78.7	78.7	78.7	78.7
8/15	49.5	59.4	69.3	79.2	89.1	99.0	104.9	104.9	104.9	104.9
8/10	-	59.4	69.3	79.2	89.1	99.0	105.1	111.1	117.0	122.8
10/20	49.5	59.4	69.3	79.2	89.1	99.0	105.1	111.1	114.2	114.2
10/15	49.5	59.4	69.3	79.2	89.1	99.0	105.1	111.1	117.0	122.8
10/10	-	60.9	69.3	79.2	89.1	99.0	105.1	111.1	117.0	122.8
12/20	-	59.4	69.3	79.2	89.1	99.0	105.1	111.1	117.0	122.8
12/15	-	60.1	69.3	79.2	89.1	99.0	105.1	111.1	117.0	122.8
12/10	-	68.8	76.3	83.4	90.2	99.0	105.1	111.1	117.0	122.8
				concrete	strength cla	ss C30/37				
8/20	54.2	65.1	75.9	78.7	78.7	78.7	78.7	78.7	78.7	78.7
8/15	54.2	65.1	75.9	86.8	97.6	104.9	104.9	104.9	104.9	104.9
8/10	-	65.1	75.9	86.8	97.6	108.4	115.1	121.7	128.2	134.5
10/20	54.2	65.1	75.9	86.8	97.6	108.4	115.1	121.7	122.9	122.9
10/15	54.2	65.1	75.9	86.8	97.6	108.4	115.1	121.7	128.2	134.5
10/10	-	65.1	75.9	86.8	97.6	108.4	115.1	121.7	128.2	134.5
12/20	-	65.1	75.9	86.8	97.6	108.4	115.1	121.7	128.2	134.5
12/15	-	65.1	75.9	86.8	97.6	108.4	115.1	121.7	128.2	134.5
12/10	-	73.1	81.0	88.6	97.6	108.4	115.1	121.7	128.2	134.5

Note: Standard dimension according to page 7 and 9. Load capacities for further rebar dimensions and for rebar spacings of 25 cm, see type test.

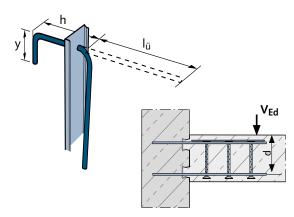
Selected Load Capacity Values according to Type Test and Approval no. Z-21.8-2035

# Shear load capacity transverse to the case — with shear reinforcement

Single connection example;  $1 \times Type 5$ 



Multipart connection example; 2 × Type 1



Please refer to the HDB Product information and approvals no. Z-15.1-249 and Z-15.1-270 when determining the shear reinforcement using HALFEN HDB-S Shear reinforcement.

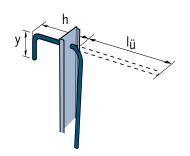
Shear load re	sistance V <sub>Rd</sub> [k	:N/m] (≤ 0.3·\	V <sub>Rd,max</sub> )				Alwa	ys refer to th	e information in	1 the type test	
rebar Ø/	d = 100 mm	d = 120 mm	d = 140 mm	d = 160 mm	d = 180 mm	d = 200 mm	d = 220 mm	d = 240 mm	d = 260 mm	d = 280 mm	
spacing s [mm/cm]	HBT 80	HBT 120	HBT 150	HBT 150	HBT 190	HBT 190	HBT 220	HBT 220	2×55(Ø8) 2×85(Ø10,12)	2×55(Ø8) 2×85(Ø10,12)	
	concrete strength class C20/25										
8/20	76.5	87.4	87.4	87.4	87.4	87.4	87.4	87.4	87.4	87.4	
8/15	76.5	102.0	116.6	116.6	116.6	116.6	116.6	116.6	116.6	116.6	
8/10	-	102.0	127.5	153.0	174.8	174.8	174.8	174.8	136.5	136.5	
10/20	76.5	102.0	109.4	109.4	109.4	109.4	109.4	109.4	109.4	109.4	
10/15	76.5	102.0	127.5	145.8	145.8	145.8	145.8	145.8	145.8	145.8	
10/10	-	102.0	127.5	153.0	178.5	204.0	218.7	218.7	218.7	218.7	
12/20	-	102.0	125.6	125.6	125.6	125.6	125.6	125.6	125.6	125.6	
12/15	-	102.0	127.5	153.0	167.5	167.5	167.5	167.5	167.5	167.5	
12/10	-	102.0	127.5	153.0	178.5	204.0	229.5	251.2	251.2	251.2	
				concrete	strength cla	ss C25/30					
8/20	87.4	87.4	87.4	87.4	87.4	87.4	87.4	87.4	87.4	87.4	
8/15	95.6	116.6	116.6	116.6	116.6	116.6	116.6	116.6	116.6	116.6	
8/10	-	127.5	159.4	174.8	174.8	174.8	174.8	174.8	158.4	158.4	
10/20	95.6	126.9	126.9	126.9	126.9	126.9	126.9	126.9	126.9	126.9	
10/15	95.6	127.5	159.4	169.2	169.2	169.2	169.2	169.2	169.2	169.2	
10/10	-	127.5	159.4	191.3	223.1	253.8	253.8	253.8	253.8	253.8	
12/20	-	127.5	145.8	145.8	145.8	145.8	145.8	145.8	145.8	145.8	
12/15	-	127.5	159.4	191.3	194.4	194.4	194.4	194.4	194.4	194.4	
12/10	-	127.5	159.4	191.3	223.1	255.0	286.9	291.5	291.5	291.5	
				concrete	strength cla	ss C30/37					
8/20	87.4	87.4	87.4	87.4	87.4	87.4	87.4	87.4	87.4	87.4	
8/15	114.8	116.6	116.6	116.6	116.6	116.6	116.6	116.6	116.6	116.6	
8/10	-	153.0	174.8	174.8	174.8	174.8	174.8	174.8	174.8	174.8	
10/20	114.8	136.6	136.6	136.6	136.6	136.6	136.6	136.6	136.6	136.6	
10/15	114.8	153.0	182.1	182.1	182.1	182.1	182.1	182.1	182.1	182.1	
10/10	-	153.0	191.3	229.5	267.8	273.2	273.2	273.2	273.2	273.2	
12/20	-	153.0	164.6	164.6	164.6	164.6	164.6	164.6	164.6	164.6	
12/15	-	153.0	191.3	219.5	219.5	219.5	219.5	219.5	219.5	219.5	
12/10	-	153.0	191.3	229.5	267.8	306.0	329.2	329.2	329.2	329.2	

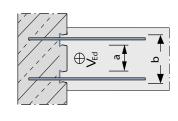
Note: Standard dimension according to page 7 and 9. Load capacities for further rebar dimensions and for rebar spacings of 25 cm, see type test.

Basis of Calculation According to Type Test and Approval no. Z-21.8-2035

# Shear load capacity, longitudinal to the case - single row profile

Multipart connection example;  $2 \times \text{Type 1}$ Standard type according to page 7





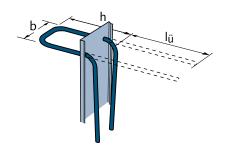
Shear load resistance V <sub>Rdi</sub> [kN/m] (≤ V <sub>Rdi,max</sub> )								Alv	ways refer t	o the inforr	nation in th	e type test	
profile	b=260 mm				<b>2</b> × <b>HBT 55</b> (Ø 8) <b>2</b> × <b>HBT 85</b> (Ø 10, 12) b=360 mm b=360 mm					b=400 mm			
rebarØ[mm]/					c	oncrete st	strength class						
spacing s [cm]	C20/25	C25/30	C30/37	C20/25	C25/30	C30/37	C20/25	C25/30	C30/37	C20/25	C25/30	C30/37	
8/20	193.4	224.4	253.4	207.4	240.7	271.8	228.5	265.1	299.4	242.5	281.4	317.7	
8/15	227.4	263.9	298.0	241.5	280.2	316.4	262.5	304.6	344.0	276.6	320.9	362.4	
8/10	295.6	343.0	387.3	309.6	359.3	405.7	330.7	383.7	433.3	344.7	400.0	451.7	
10/20	213.7	248.0	280.0	227.7	264.2	298.4	248.8	288.7	326.0	262.8	304.9	344.3	
10/15	254.5	295.3	333.5	268.5	311.6	351.9	289.6	336.0	379.5	303.6	352.3	397.8	
10/10	336.2	390.1	440.5	350.2	406.4	458.9	371.3	430.8	486.5	385.3	447.1	504.9	
12/20	231.9	269.1	303.9	245.9	285.4	322.2	267.0	309.8	349.8	281.0	326.1	368.2	
12/15	278.8	323.5	365.3	292.8	339.8	383.7	313.9	364.2	411.3	327.9	380.5	429.6	
12/10	372.6	432.3	488.2	386.6	448.6	506.6	407.7	473.0	534.2	421.7	489.3	552.6	

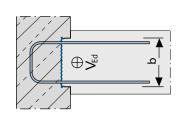
Note: Load capacities for further joint widths, for further rebar dimensions and for rebar spacings of 25 cm, see type test.

The joint areas a between the HBT Cases must be designed as rough or suitably indented as defined in DIN EN 1992-1-1; 6.2.5.

# Shear load capacity, longitudinal to the case - double-row profile

Single connection example; 1 × **Type 5** Standard type according to page 9



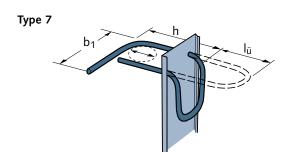


Shear load resistance V <sub>Rdi</sub> [kN/m] (≤ V <sub>Rdi,max</sub> ) for HBT 120, 150, 190 and 220							Always refer to the information in the type te						
profile	HBT 120 HBT 150				HBT 190				HBT 220				
rebarØ[mm]/							rength clas						
spacing s [cm]	C20/25	C25/30	C30/37	C20/25	C25/30	C30/37	C20/25	C25/30	C30/37	C20/25	C25/30	C30/37	
8/20	145.0	168.2	190.0	154.8	179.6	202.9	167.4	194.3	219.4	180.1	208.9	235.9	
8/15	179.1	207.8	234.6	188.9	219.2	247.5	201.5	233.8	264.0	214.1	248.5	280.6	
8/10	247.2	286.8	323.9	257.0	298.2	336.8	269.6	312.9	353.3	282.3	327.5	369.9	
10/20	165.3	191.8	216.6	175.1	203.2	229.5	187.7	217.8	246.0	200.4	232.5	262.5	
10/15	206.1	239.2	270.1	215.9	250.6	283.0	228.6	265.2	299.5	241.2	279.9	316.0	
10/10	279.0	323.8	365.6	297.6	345.3	390.0	310.2	360.0	406.5	322.9	374.6	423.1	
12/20	183.5	212.9	240.4	193.3	224.3	253.3	205.9	239.0	269.8	218.6	253.6	286.4	
12/15	230.4	267.3	301.9	240.2	278.7	314.8	252.8	293.4	331.3	265.5	308.0	347.8	
12/10	306.3	355.5	401.4	334.0	387.6	437.7	346.6	402.2	454.2	359.3	416.9	470.8	

Note: Load capacities for the HBT 80 profile; for further rebar dimensions and for rebar spacings of 25 cm, see type test

# Calculating Reinforced Concrete Corbel

# Calculating a reinforced concrete corbel for HBT Type 7



# Strutural boundary conditions

 $0.2 \le \frac{a_C}{h_C} \le 1.0$ Geometric assumptions:

Anchorage length in the corbel:

$$I_{bd,dir} = \frac{2}{3} I_{b,eq} \ge \max \quad \begin{cases} 0.67 \cdot \alpha_1 \cdot \alpha_4 \cdot I_{b,rqd} \\ 6.7 \emptyset \end{cases}$$

# Shear load capacity in the corbel:

$$F_{Ed} \le V_{Rd,max} = 0.5 \cdot v \cdot b_c \cdot z \cdot \frac{f_{ck}}{\gamma_c}$$
where  $v = 0.7 - \frac{f_{ck}}{200 \text{ N/mm}^2} \ge 0.5$ 
 $z = 0.9 \cdot d$ 

# Tensile load in the corbel:

$$\begin{split} &Z_{Ed} = F_{Ed} \cdot \frac{a_C}{z_O} + H_{Ed} \cdot \frac{a_H + z_O}{z_O} \\ &\text{where } z_O = d \cdot \left(1 - 0.4 \cdot \frac{F_{Ed}}{V_{Rd,max}}\right); \; H_{Ed} \geq 0.2 \cdot F_{Ed} \\ &\text{and } \frac{a_C}{z_O} \; \geq 0.4 \end{split}$$

#### Required reinforcement for tension

$$a_{s,rqd} = min \begin{cases} \frac{Z_{Ed}}{0.8 \cdot f_{yd}}; \text{ where } F_{bd} = \text{anchored load} \\ \frac{F_{bd}}{0.8 \cdot f_{yd}} \end{cases}$$

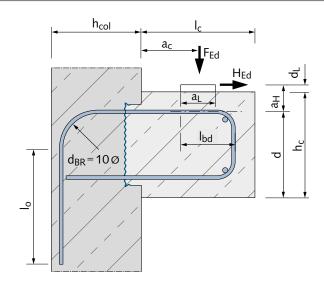
Verifying the reinforcement overlap length in the wall for:

$$I_{0,avail.} = b_1 - 6\emptyset$$

In addition to the corbel calculation above, the rebend connection has to be verified according to the specifications in approval no. Z-21.8-2035 (see page 10 f).

The compressive stresses under the load application plate and the wall's joint load bearing capacity must be verified in accordance with DIN EN 1992-1-1 resp. publication no. 532 issued by the German Committee for Structural Concrete (DAfStb).

The spacing between the side-edge of the corbel and the outermost stirrups in the HBT Connection should not exceed 5 cm. The free side-edge of the corbel must be strengthened using stirrups.



			h		
			11(	;=	
		160 mm	190 mm	230 mm	250 mm
	reinforce- ment	max	F <sub>Ed</sub> for prof	file type [kN/	/m]
	Ø/s	HBT 120	HBT 150	HBT 190	HBT 220
$L_c = 200 \text{ mm}$	8/25	51.7	61.7	61.4	61.3
$I_{\ddot{u}} = 165 \text{ mm}$	8/20	64.2	76.9	77.1	77.0
$a_c = 100 \text{ mm}$	8/15	64.5	-	-	-
	10/25	63.7	76.4	91.8	96.7
	10/20	64.0	80.1	101.5	110.1
	10/15	-	-	101.5	110.1
$L_c = 220 \text{ mm}$	8/25	53.4	61.5	61.2	61.1
$I_{\ddot{u}} = 185 \text{ mm}$	8/20	64.4	77.3	77.0	76.8
$a_c = 110 \text{ mm}$	10/25	63.9	79.1	95.4	96.5
	10/20	-	79.9	101.3	110.0
	12/25	63.3	79.4	100.8	109.6
	12/20	-	79.4	100.8	109.6
$L_c = 240 \text{ mm}$	8/25	53.3	61.4	61.1	60.9
$I_{\ddot{u}} = 205 \text{ mm}$	8/20	64.3	77.1	76.8	76.7
$a_c = 120 \text{ mm}$	10/25	63.8	79.8	96.5	96.3
	10/20	-	-	101.2	109.8
	12/25	63.2	79.3	100.6	109.5
$L_c = 260 \text{ mm}$	8/25	50.1	60.5	60.9	60.7
$I_{\ddot{u}} = 225 \text{ mm}$	8/20	62.2	-	-	-
$a_c = 130 \text{ mm}$	10/25	63.7	79.7	96.3	96.2
	12/25	-	79.1	100.5	109.3
$L_c = 280 \text{ mm}$	8/25	47.3	57.2	60.8	60.6
$l_{\ddot{u}} = 245 \text{ mm}$	10/25	-	-	96.2	96.0
$a_c = 140 \text{ mm}$	12/25	-	79.0	-	-

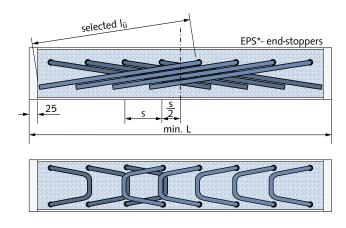
 $H_{Ed} = 0.2 \cdot F_{Ed}$ ; h = 170 mm, b<sub>1</sub> = 400 mm load-plate a<sub>L</sub> = 50 mm, d<sub>L</sub> = 10 mm

These values are for pre-dimensioning; final values must be verified. Further dimensions and resistance values are available on request. Generally, significantly better resistance values are possible when using the HALFEN HSC Stud connector.



#### Layout of the Reinforcement in the Case

# Minimal element lengths for the rebend connections - layout of the reinforcement bars in the case



*	Expanded	poly	∕st\	/rene

Minimal element lengths for l <sub>ü</sub> standard									
Ø [mm]	rebar spacing s [cm]	① l <sub>ü</sub> [mm]	minimal case length L [mm]	number of rebar					
8	10	320	650	6					
8	15	320	600	4					
8	20	320	650	4					
10	10	390	800	8					
10	15	390	750	4					
10	20	390	700	4					
12	10	460	950	8					
12	15	460	900	6					
12	20	460	850	4					
① Max. lii - observe dimensions for selected profile (see page 7 and 9)									

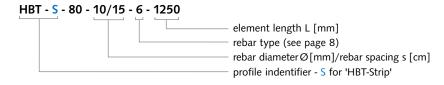
# HBT Strip — the perfect solution for precast elements

Due to inherent low concrete cover and restriction in element size in precast elements, demands on precise measurement and manufacturing of rebend connections are generally higher.

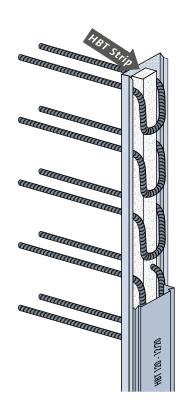
The HBT-Connection with 'HBT Strip' (HBT-S) fulfils these requirements perfectly. Available for all double-row HBT connections with rebar types 6, 7, 8 and 9, in profile widths HBT - S 80, 120, 150, 190, 220.

- · rebars are secured during transport
- compliance with required over-lap and anchorage lengths
- recommended for loops and corbel connections
- HBT Strip is easily removed after striking the formwork

#### Order example:



Installation is as described on page 18; the HBT Strip must be removed after all connecting rebar have been rebent.



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**Application Suggestions** 

#### Application of short elements

#### Various lengths - efficient installation times

Combining 1250 mm standard elements and 800 mm elements helps to avoid unnecessary modification of HBT Elements.

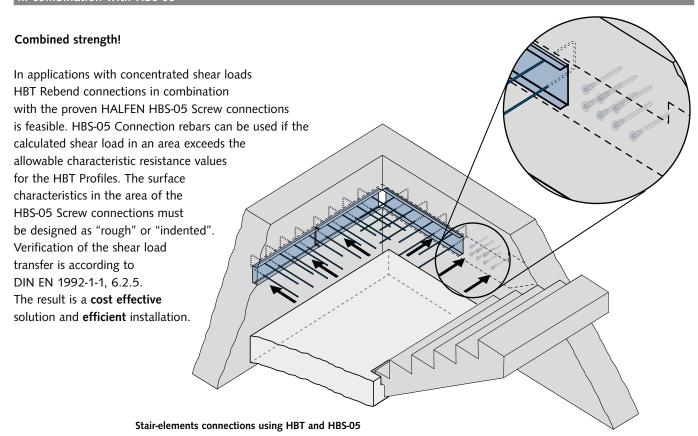
Standard combinations								
	clear floor height [m]							
HBT Element	2.40	2.50	2.85	3.30	3.75			
	combinations							
short element L = 800 mm	3	-	2	1	-			
standard element L = 1250 mm	-	2	1	2	3			

In numerous applications, further on-site modification of the HBT cases is therefore not required. The risk of damaging the rebar in the casing is avoided. The planner can plan more efficiently and on-site preparation time for installation of the HBT Elements is reduced.

1250 mm

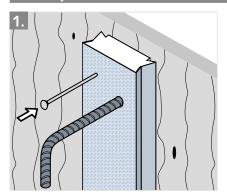
Wall connection using two 1250 mm HBT Elements and a 800 mm HBT Element to obtain 3.30 m floor height

#### In combination with HBS-05

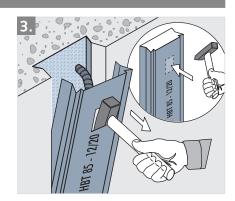


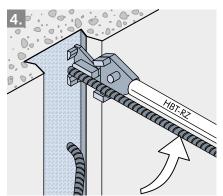
#### Assembly Instructions

#### Assembly instructions









- Nail the HBT Element to the timber formwork in the specified position. Use suitable methods for fixing elements to metal formwork, for example magnets. Check stirrup lengths protruding from the case after installation.
- After the concrete has cured, strike the formwork; hitting a wood block placed in the long groove in the cover with a hammer, loosen the cover.
- Use a hammer with a claw to punch-in the perforated hole in the cover; hook the hammer in the hole and pull the lid out.
- Place the Rebending tool under the rebar and pull down on the handle evenly with both hands until the rebar is in the correct position. The bar must be rebent straight without any kinks. Proceed in a similar manner to rebend all bars in the HBT Element one-by-one. The profiled back of the HBT Element case remains in the concrete.

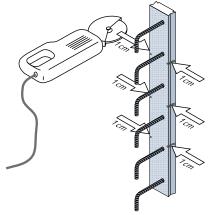
Note: A more detailed installation instruction can be found at www.halfen.com.

### HBT Element adapted to curved formwork

# Adapting the HBT Case

Using an angle-grinder cut approximately 1 cm deep incisions symmetrically into both sides of the case at regular intervals; the HBT housing loses its rigidity, easing fixing to the formwork. To achieve a better fit to smaller curvature (< 3.00 m), up to seven incisions per side are possible.

After fixing the HBT Case to the formwork cover the incisions with adhesive tape to prevent concrete seeping into the form.



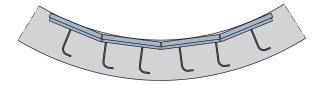


Caution when working with an angle-grinder! The reinforcing steel bars in the HBT Case must not be damaged.



HBT Element fitted to an convex curvature

Outer radius ≥ ca. 3.00 m; smaller radius is achieved with more incisions.



HBT Element fitted to an concave curvature Inner radius ≥ ca. 3.00 m; smaller radius is achieved with more incisions.

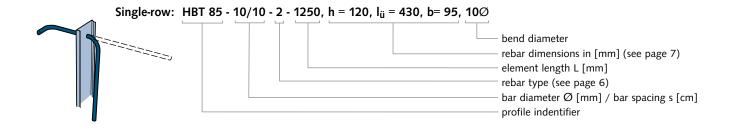
#### Order Examples

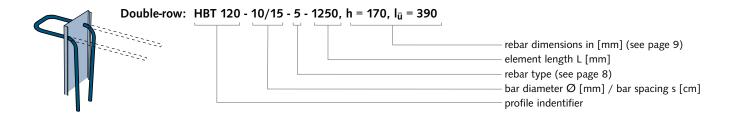
# Order examples

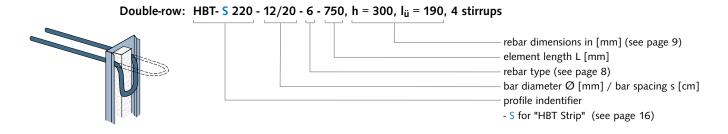
We offer a wide range of standard HBT Rebend connections for the most common applications (see page 6ff.).

There is also a wide product range with corresponding profile widths and rebar shapes to choose from.

Rebar dimensions and element lengths are freely definable, limited only by geometric specifications and limits in production (see also page 16).



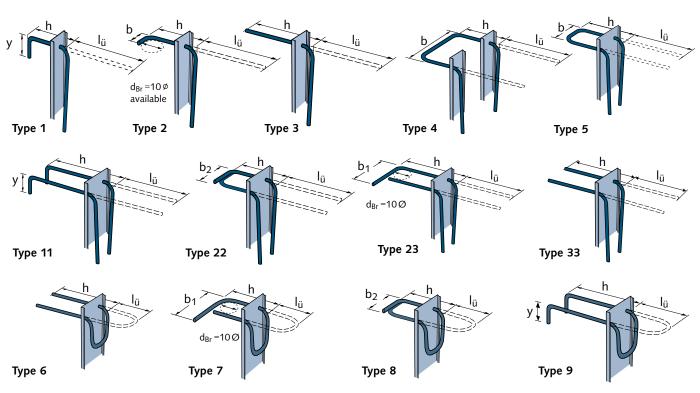




HBT Connection with foam strip filler "HBT Strip" (see page 16).

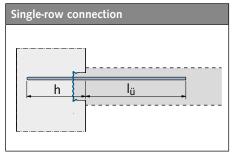
# Order form

# Single and double-row connections

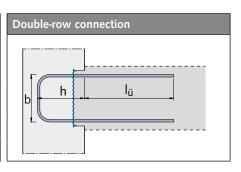


Pos.	profile HBT	rebar Ø [mm]	rebar spacing <b>s</b> [cm]	stirrup type	element length <b>L</b> [mm]	<b>h</b> [mm]	l <sub>ü</sub> [mm]	1 × v	/alue if pre	in[mi esent	m] <b>b</b> <sub>2</sub>	$d_{Br}$	total length [m]	number of elements [items]

Note: do not exceed  $l_{\ddot{u},max}$  see table on page 7 and 9; see page 6 to 9 for rebar and profile dimensions.



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Tel. no.	See inner
E-Mail	back-cover
Fax	for addresses.



Tender Texts

# HALFEN Rebend connection, type HBT 85-10/20-1-1250

HALFEN HBT Rebend connection with single-row rebend reinforcement in a galvanised sheet-metal case to form a reinforced connection, with general building authority approval no. Z-21.8-2035,

```
Type HBT 85 – 10/20 – 1 – 1250
with
85 = type identifier for a case width of 85 mm with a single-row of rebar,
10/20 = reinforcement steel B500B with 10 mm bar diameter and 200 mm bar spacing,
1 = standard rebar type 1,
1250 = element length [mm],
in standard rebar dimensions
rebar length 1 h = 170 mm,
rebar length 2 | I<sub>ü</sub> = 390 mm,
bend length y = 95 mm,
```

or equivalent; deliver and install according to the manufacturer's instructions.

# HALFEN Rebend connection, type HBT 150-12/15-5-1250

HALFEN HBT Rebend connection with double-row rebend reinforcement in a galvanised sheet-metal case to form a reinforced connection, with general building authority approval no. Z-21.8-2035,

```
Type HBT 150 - 12/15 - 5 - 1250
with

150 = type identifier for a case width of 150 mm with a double-row of rebar,

12/15 = reinforcement steel B500B with 12 mm bar diameter and 150 mm bar spacing,

5 = standard rebar type 5,

1250 = element length [mm],

in standard rebar dimensions

rebar length 1 h = 170 mm,

rebar length 2 l<sub>ü</sub> = 460 mm,
```

or equivalent; deliver and install according to the manufacturer's instructions.

# HALFEN Rebend connection, with stainless steel reinforcement bars

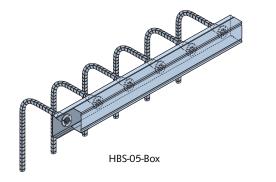
Use the text as above, but replace "B500B" with "stainless steel B500 NR according to building authority approval".

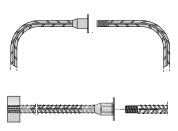
Further tender texts can be found at www.halfen.com.

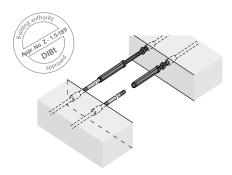
#### Further HALFEN Reinforcement Products

# HALFEN HBS-05 Screw connections

The HALFEN HBS-05 Screw connections allow rebar continuity joints possible with simple screw and socket rebar. Their versatility allows nearly every type of reinforcement joint. HALFEN HBS-05 fulfils German and international certification criterion. Extensive certification and test reports prove their suitability also under extreme conditions.





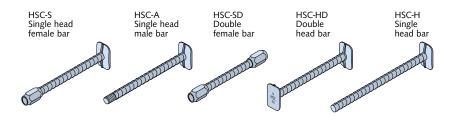


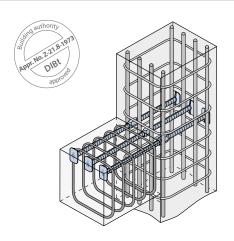
No torque wrench or special tools required – visual check is sufficient

#### HALFEN HSC Stud Connector

The HALFEN HSC Stud connector is a building authority approved reinforcement optimized for anchorage in concrete. Maximum exploitation of the reinforcement is possible with extremely short anchoring lengths.

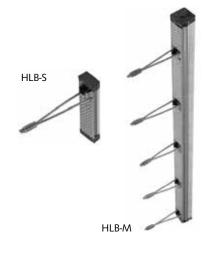
HALFEN HSC Anchor is especially advantageous for use in high-density reinforced areas, for example; corbels and frame corner nodes.

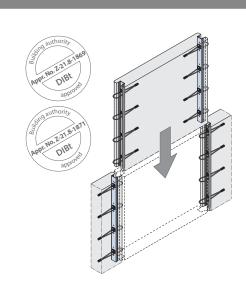




#### HALFEN HLB Loop Box

The HALFEN HLB Loop Box is an efficient and time saving method for connecting concrete elements. HLB Loop boxes are cast into concrete elements (e.g. wall elements) in the precast plant. The prepared concrete elements are transported to site, lifted and correctly placed with a crane; the joint between the elements is then cement-grouted.





#### References



Kö-Bogen, Düsseldorf, Germany

The Kö-Bogen complex designed by Daniel Libeskind offers commercial space for business as well as a shopping centre with restaurants etc. The Kö-Bogen (curve) is located at the end of the Königsallee in Düsseldorf, Germany.

We supplied HALFEN HBT Rebend connections for the Kö-Bogen. Other HALFEN products also used in this project include;

- HALFEN HTA Channels
- HALFEN Masonry connection channels
- HALFEN HDB Shear rails
- HALFEN HBS-05 Screw connections



Kopernikus Science centre, Warsaw, Poland

The Science centre in Warsaw, Poland, named after Nikolaus Kopernikus, is a collection of buildings with six exhibition areas.

We supplied HALFEN HBT Rebend connections for this project.



DATEV IT-Campus, Nuremberg, Germany

The DATEV IT-Campus on Fürther Strasse in Nuremberg offers 1800 workspaces including 200 individual offices and conference rooms of various sizes. The building was inaugurated in 2015.

We supplied HDB Shear rails and HBT Rebend connections for this project.



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